

STRUCTURAL NOTES STRUCTURAL STEEL SPECIAL INSPECTIONS POST-INSTALLED ANCHORS THE FOLLOWING ELEMENTS OF CONSTRUCTION SHALL HAVE CONTINUOUS INSPECTION 1. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING STRENGTH DESIGN CAPACITIES (SEISMIC) FOR CARBON HILTI KWIK-BOLT TZ IN CONCRETE. 1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED BY AN BY A BUILDING INSPECTOR APPROVED BY CITY OF LOS ANGELES. CONSTRUCTION. THE ARCHITECT SHALL BE NOTIFIED OF ANY (TYPICAL INSTALLATIONS PER ICC REPORT ESR-1917) APPROVED AND LICENSED FABRICATOR IN ACCORDANCE WITH THE AISC DISCREPANCIES OR INCONSISTENCIES. CONCRETE. SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF 2. BOLTS INSTALLED IN CONCRETE. CARBON ALL PSI STRUCTURAL STEEL FOR BUILDINGS (2010 EDITION), AND WITH CHAPTER 4000psi **TORQUE** 2. ALL DRAWINGS ARE CONSIDERED TO BE A PART OF THE CONTRACT 3. PLACING OF REINFORCING STEEL 22 OF THE CODE. DOCUMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND 4. ALL STRUCTURAL WELDING, INCLUDING REINFORCING STEEL. COORDINATION OF ALL DRAWINGS AND SPECIFICATIONS PRIOR TO THE START 2. ALL STRUCTURAL STEEL SHALL CONFORM TO THE ASTM DESIGNATION AS HIGH STRENGTH BOLTING. VALUE OF CONSTRUCTION. ANY DISCREPANCIES THAT OCCUR SHALL BE BROUGHT TO INDICATED BELOW (UNO): 1,2,3,4,5,6 1,2,3,4,5,6 6. EXPANSION ANCHOR. THE ATTENTION OF THE ARCHITECT PRIOR TO START OF CONSTRUCTION SO THAT A CLARIFICATION CAN BE ISSUED. ANY WORK PERFORMED IN WF SHAPES A992 OR A572, GR50 1,212 1,099 CONFLICT WITH THE CONTRACT DOCUMENTS OR ANY CODE REQUIREMENTS PLATES (AS NOTED ON DRAWINGS) SHALL BE CORRECTED BY THE CONTRACTOR AT THEIR OWN EXPENSE AND AT NO EXPENSE TO THE OWNER OR ARCHITECT. 1. ALL CONCRETE CONSTRUCTION SHALL CONFORM WITH CHAPTER 19 OF THE CONNECTION PLATES, AND MISC. A-36 U.N.O. CODE AND WITH THE PROVISIONS OF ACI 318-11 3 1/4 2,625 NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL A-500, GRADE B HOLLOW STRUCTURAL SECTIONS NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE GIVEN, 2. REINFORCED CONCRETE IS DESIGNED BY THE "ULTIMATE STRENGTH DESIGN CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK. A325N UNO 3 1/8 2,508 3. CONCRETE MIXES SHALL BE DESIGNED BY THE APPROVED TESTING LABORATORY 4. ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE FOLLOWING F1554, GR 36 U.N.O. **BOLTS IN CONCRETE** AND APPROVED BY THE STRUCTURAL ENGINEER. THE COMPRESSIVE STRENGTH OF 5,136 THE CONCRETE SHALL BE PROPORTIONED BASED ON SECTION 1905 OF THE CODE. ANGLES, CHANNELS, WT SHAPES 2012 INTERNATIONAL BUILDING CODE REFERRED TO HERE AS "THE CODE", AND ANY 4. SCHEDULE OF STRUCTURAL CONCRETE 28-DAY STRENGTH AND TYPES: 3 3/4 THE STRUCTURAL STEEL FABRICATOR SHALL FURNISH SHOP DRAWINGS TO THE REGULATING AGENCIES WHICH HAVE AUTHORITY OVER ANY PORTION OF THE WORK, ARCHITECT OF ALL STEEL FOR ARCHITECTS AND STRUCTURAL ENGINEERS LOCATION IN STRUCTURE STRENGTH DENSITY REVIEW AND APPROVAL BEFORE FABRICATION. 4 3/4 THE DEPARTMENT OF VETERAN AFFAIRS STANDARD DVA H-18-8. (PSI) (PCF) 4. BOLT HOLES USED IN STEEL SHALL BE 1/16" LARGER IN DIAMETER THAN 5. SEE ARCHITECTURAL DRAWINGS FOR THE FOLLOWING: ALL CONC (NWT) NOMINAL SIZE OF BOLT USED, EXCEPT AS NOTED. SIZE AND LOCATION OF ALL DOOR AND WINDOW OPENINGS, EXCEPT AS 5. ALL STRUCTURAL STEEL SURFACES THAT ARE ENCASED IN CONCRETE, OR 5. PORTLAND CEMENT SHALL CONFORM TO ASTM C-150, TYPE II. MASONRY, SPRAY ON FIREPROOFING, OR ARE ENCASED BY BUILDING FINISH, SHALL BE LEFT UNPAINTED. 1. VALUES ARE FOR SINGLE ANCHORS WITH MINIMUM SPACING OF 3hef BETWEEN ADJACENT ANCHORS AND 6. AGGREGATE FOR HARDROCK CONCRETE SHALL CONFORM TO ALL REQUIREMENTS SIZE AND LOCATION OF ALL INTERIOR AND EXTERIOR NON-BEARING MINIMUM EDGE DISTANCE OF 1.5hef OR Ccr. (NO EDGE DISTANCE OR SPACING REDUCTION). FOR AND TESTS OF ASTM C-33 AND PROJECT SPECIFICATIONS. EXCEPTIONS MAY PARTITIONS. ALL WELDING IS TO BE DONE BY CERTIFIED WELDERS USING E70XX GROUPS OF ANCHORS AND/OR SPACING AND EDGE DISTANCES LESS THAN INDICATED REFER TO BE USED ONLY WITH PERMISSION OF THE STRUCTURAL ENGINEER. ELECTRODES (UNO). ALL WELDS SHALL BE IN CONFORMITY WITH THE SIZE AND LOCATION OF ALL CONCRETE CURBS, EQUIPMENT PADS, PITS, PROJECT SPECIFICATIONS AND THE CODE FOR WELDING IN BUILDING FLOOR DRAINS, SLOPES, DEPRESSED AREAS, CHANGE IN LEVEL, CHAMFERS, 7. CONCRETE MIXING OPERATION, ETC. SHALL CONFORM TO ASTM C-94. CONSTRUCTION (AWS D1.1 LATEST REVISION) OF THE AMERICAN WELDING 2. TENSION VALUES ØNn ARE FOR NORMAL WEIGHT CONCRETE, FOR LIGHT-WEIGHT CONCRETE MULTIPLY VALUES BY 0.60. GROOVES, INSERTS, ETC. SOCIETY. SEE SPECIAL INSPECTIONS SECTION FOR WELDING INSPECTION 8. PLACEMENT OF CONCRETE SHALL CONFORM TO CODE SECTION 1905 AND 3. Ø FACTORS FOR TENSION ARE THE REDUCTION FACTORS FOR CONDITION B WHERE SUPPLEMENTARY REQUIREMENTS. PROJECT SPECIFICATIONS. CLEAN AND ROUGHEN TO 1/4" AMPLITUDE ALL SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS EXCEPT AS SHOWN. REINFORCEMENT IN CONFORMANCE WITH ACI 318-11 SECTION D.4.4 IS NOT PROVIDED OR WHERE PULLOUT CONCRETE SURFACES AGAINST WHICH NEW CONCRETE IS TO BE PLACED. WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH OR PRYOUT STRENGTH GOVERNS. DIA. TENSION= 0.65. REQUIRED. WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF FLOOR AND ROOF FINISHES. 9. ALL REINFORCING BARS, ANCHOR BOLTS AND OTHER CONCRETE INSERTS SIZE, USE MINIMUM SIZE WELDS AS SPECIFIED IN AISC MANUAL OF STEEL 4. Ø FACTOR FOR SHEAR ARE THE REDUCTION FACTORS FOR STEEL FAILURE MODES OF A DUCTILE STEEL SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE. CONSTRUCTION, SECTION J2. DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS. ELEMENT AS DEFINED BY ACI 318 SECTION D.1. Ø SHEAR= 0.70. 8. ALL EXPOSED STRUCTURAL STEEL AND MISCELLANEOUS METAL SHALL BE HOT 6. SEE MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS FOR THE 10. PROVIDE SLEEVES FOR PLUMBING AND ELECTRICAL OPENINGS IN CONCRETE 5. THE VALUES LISTED ARE FOR USE WITH THE LOAD COMBINATIONS OF 2012 IBC. DIP GALVANIZED AFTER FABRICATION. BEFORE PLACING. DO NOT CUT ANY REINFORCING WHICH MAY CONFLICT. CORING IN CONCRETE IS NOT PERMITTED. NOTIFY THE STRUCTURAL 9. THE USE OF E70-T4 WELDING WIRE IS NOT ALLOWED FOR ANY APPLICATION. 6. ALL VALUES LISTED FOR CRACKED CONRETE USING Kcr AS LISTED IN TABLES 3 & 4 OF ICC ESR 1917 PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ENGINEER IN ADVANCE OF CONDITIONS NOT SHOWN ON THE DRAWINGS. SEE ETC., EXCEPT AS SHOWN OR NOTED. 10. ALL WELD FILLER MATERIAL SHALL HAVE A MINIMUM CHARPY V-NOTCH (CVN) 7. TABLE APPLIES TO KWIK BOLT TZ ANCHORS INSTALLED IN CONCRETE SLAB, WALLS, BEAMS. THESE DRAWINGS FOR ADDITIONAL RESTRICTIONS ON THE PLACEMENT OF VALUE OF 20 FT-LBS AT A TEMPERATURE OF O°F. OPENINGS IN SLABS AND WALLS. ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS. 8. VALUES LISTED ARE FOR ANCHORS SUBJECT TO SEISMIC LOADS. FOR ANCHORS SUBJECT TO STATIC 11. 100 PERCENT ULTRASONIC TESTING FOR ALL COMPLETE JOINT PENETRATION LOADS ONLY, SEE ICC ESR-1917. CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL OR PLUMBING FIXTURES. 11. CONDUITS AND PIPE SLEEVES SHALL NOT DISPLACE OR INTERRUPT GROOVE WELDS IS REQUIRED. REINFORCING BARS, SPACE EMBEDDED CONDUITS AND SLEEVES AT A 9. ANCHOR MAY BE TESTED TO THE FOLLOWING: SIZE AND LOCATION OF MACHINE OR EQUIPMENT AND EQUIPMENT PADS MINIMUM CLEAR SPACING OF 3 DIAMETERS. THE TORQUE TEST VALUE LISTED IN TABLE ABOVE BY USE OF A CALIBRATED TORQUE WRENCH. 7. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF STEEL DECK 10. WHEN INSTALLING TZ ANCHORS, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY REINFORCING BARS. TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL REINFORCING STEEL (FOR CONCRETE) INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO 1. ROOF AND FLOOR DECKS SHALL BE AS NOTED ON THE DRAWINGS. PROPERTIES 11. APPLY PROOF TEST LOADS TO ANCHORS WITHOUT REMOVING THE NUT IF POSSIBLE. IF NOT, REMOVE CONSTRUCTION EQUIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY 1. REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 19 NUT AND INSTALL A THREADED COUPLER TO THE SAME TIGHTNESS OF THE ORIGINAL NUT USING A ARE AS FOLLOWS: THE STRUCTURAL ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE OF THE CODE, ASTM A706, GRADE 60 UNO. TORQUE WRENCH AND APPLY LOAD. ITEMS. DEFORMATIONS SHALL BE IN ACCORDANCE WITH ASTM A-305. DECK SIZE AND GAUGE 12. TEST EQUIPMENT IS TO BE CALIBRATED BY AN APPROVED TESTING LABORATORY IN ACCORDANCE WITH 8. OPENINGS, POCKETS, ETC., LARGER THAN 6" SHALL NOT BE PLACED IN 0.322 1-1/2" x 18 GA DECK 2. BARS SHALL BE CLEAN OF RUST, GREASE, OR OTHER MATERIALS LIKELY TO STANDARD RECOGNIZED PROCEDURES. CONCRETE SLABS, DECKS, WALLS, UNLESS SPECIALLY DETAILED ON THE IMPAIR BOND. ALL REINFORCING BAR BENDS SHALL BE MADE COLD. STRUCTURAL DRAWINGS. NOTIFY THE STRUCTURAL ENGINEER WHEN DRAWINGS 13. THE FOLLOWING CRITERIA APPLY FOR THE ACCEPTANCE OF INSTALLED ANCHORS: BY OTHERS SHOW OPENINGS, POCKETS, ETC., LARGER THAN 6" NOT SHOWN 3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. PROVIDE LAPS AS ON THE STRUCTURAL DRAWINGS, BUT WHICH ARE LOCATED IN STRUCTURAL PER THE CODE, 9" MINIMUM. WWF SHALL BE SUPPORTED 2. DECK SHOP DRAWINGS SHALL BE SUBMITTED TO ENGINEER FOR APPROVAL PRIOR a. HYDRAULIC RAM METHOD: ANCHORS TESTED WITH A HYDRAULIC JACK OR SPRING LOADED MEMBERS. FOR ANY FURTHER RESTRICTIONS ON OPENINGS IN STRUCTURAL ON APPROVED CHAIRS. TO FABRICATION AND SHALL INDICATE STUD LAYOUT. DEVICES SHALL MAINTAIN THE TEST LOAD FOR A MINIMUM OF 15 SECONDS AND SHALL ELEMENTS, SEE APPLICABLE SECTIONS BELOW. EXHIBIT NO DISCERNABLE MOVEMENT DURING THE TENSION TEST, e.g., AS EVIDENCED BY 4. REINFORCING BAR SPLICES SHALL BE MADE AS INDICATED ON THE THE AMERICAN IRON AND STEEL INSTITUTE "SPECIFICATIONS FOR THE DESIGN OF LOOSENING OF THE WASHER UNDER THE NUT. LIGHT GAUGE STEEL STRUCTURAL MEMBERS" SHALL GOVERN THE DESIGN OF ALL DRAWINGS. MINIMUM SPLICE LENGTH FOR REINFORCING STEEL BARS IN 9. OPENINGS, POCKETS, ETC., SMALLER THAN 6" MAY BE PLACED IN EXISTING DECK UNITS, STEEL DECK AND ALL OF ITS CLOSURES AND FLASHINGS SHALL CONCRETE SHALL BE AS PER THE CODE. LAP ALL HORIZONTAL BARS AT CONCRETE SLABS, DECKS, WALLS, PROVIDED NO EXISTING REINFORCEMENTS b. TORQUE WRENCH METHOD: ANCHORS TESTED WITH A CALIBRATED TORQUE WRENCH CONFORM TO ASTM A653 SS, GRADE 33, Fy = 38,000 PSI MINIMUM. ARE CUT. MULTIPLE OPENINGS SHALL NOT BE PLACED CLOSER THAN 36" CORNERS AND INTERSECTIONS. STAGGER ALL SPLICES UNLESS NOTED MUST ATTAIN THE SPECIFIED TORQUE WITHIN 1/2 TURN OF THE NUT. O.C. USE NON-DESTRUCTIVE METHODS TO LOCATE EXISTING REINFORCING OTHERWISE ON PLANS. BARS. DO NOT CUT EXISTING REINFORCING BARS WHICH MAY CONFLICT WITH 4. ACCEPTABLE STEEL DECK MANUFACTURERS ARE AS FOLLOWS: 14. TESTING SHOULD OCCUR 24 HOURS MINIMUM AFTER INSTALLATION OF THE SUBJECT ANCHORS. 5. ALL BARS SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN OPENING LOCATION UNLESS SPECIFICALLY APPROVED BY THE ENGINEER. VERCO MANUFACTURING INC. (ICC ESR-1735P) THE FINAL IN-PLACE INSPECTION IS MADE. 15. SPECIAL INSPECTION IN ACCORDANCE WITH TABLE 1705.3 OF THE IBC SHALL BE PROVIDED ASC STEEL DECK (ICC ESR-1414) 10. CONDUITS LARGER THAN 1-1/2" DIAMETER SHALL NOT BE EMBEDDED IN FOR ANCHOR INSTALLATIONS. 6. WHERE WELDING OF REINFORCING IS APPROVED BY THE STRUCTURAL STRUCTURAL CONCRETE EXCEPT WHERE SPECIFICALLY APPROVED. NO ENGINEER, IT SHALL BE DONE BY AWS CERTIFIED WELDERS USING E90XX OR 5. UNITS SHALL BE CONTINUOUS OVER THREE OR MORE SPANS, EXCEPT WHERE THE 16 FOR ANCHORS BY BRANDS OTHER THAN THAT LISTED ON THESE DRAWINGS, CONTRACTOR SHALL CONDUITS SHALL BE PLACED IN CONCRETE FILL OVER METAL DECKING APPROVED ELECTRODES. WELDING PROCEDURES SHALL CONFORM TO THE FRAMING DOES NOT PERMIT. SHORING MAY BE REQUIRED AT NON-CONTINUOUS SUBMIT APPROVED TEST REPORTS FOR ENGINEER'S APPROVAL PRIOR TO INSTALLATION, FOR THE AND NO PIPES SHALL BE EMBEDDED IN CONCRETE. REQUIREMENTS OF STRUCTURAL WELDING CODE- REINFORCING STEEL". PARTICULAR LOCATION OF USE. SPANS. DECK SHOP DRAWINGS SHALL INDICATE WHERE SHORING WILL BE AWS-D1.4, LATEST REVISION. REINFORCING BARS TO BE WELDED SHALL REQUIRED. DECK SHALL BEAR A 2" MINIMUM AT ALL SUPPORTS. ALL DECKS FOR 11. ASTM SPECIFICATIONS ON THE DRAWINGS SHALL BE OF THE LATEST CONFORM TO THE REQUIREMENTS OF ASTM A-706. 17. USE NON-DESTRUCTIVE METHODS TO LOCATE EXISTING REINFORCING BARS. DO NOT CUT EXISTING INTERIOR FLOORS SHALL HAVE VENT TABS FOR CONCRETE VENTILATION. REINFORCING BARS WHICH MAY CONFLICT WITH ANCHOR LOCATION UNLESS SPECIFICALLY APPROVED 7. BARS IN SLABS SHALL BE SECURELY SUPPORTED ON WELL-CURED CONCRETE BY THE ENGINEER. 6. ALL WELDING OF STEEL DECK SHALL BE DONE BY CERTIFIED LIGHT GAGE BLOCKS OR APPROVED METAL CHAIRS, PRIOR TO PLACING CONCRETE. 12. CONTRACTOR SHALL INVESTIGATE SITE DURING CLEARING AND EARTHWORK WELDERS IN ACCORDANCE WITH AWS "SPECIFICATIONS FOR WELDING SHEET OPERATIONS FOR FILLED EXCAVATIONS OR BURIED STRUCTURES, SUCH AS 8. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE "A.C.I. STEEL IN STRUCTURES", AWS D1.3, LATEST EDITION. CESSPOOLS, CISTERNS, FOUNDATIONS, ETC. IF ANY SUCH STRUCTURES ARE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE FOUND, STRUCTURAL ENGINEER SHALL BE NOTIFIED IMMEDIATELY. STRUCTURES", LATEST EDITION. 7. UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE END OF THE UNITS AND AT INTERMEDIATE SUPPORTS AND TO THE STEEL SUPPORTS AT THE SIDE 9. COMPLETE AND DETAILED REINFORCING PLACEMENT DRAWINGS SHALL BE DRILLED ADHESIVE ANCHORS 13. CONSTRUCTION MATERIAL SHALL BE SPREAD OUT IF PLACED ON FRAMED ROOF BOUNDARIES BY 3/4" DIAMETER PUDDLE WELDS AT 1'-0" OC. PREPARED AND SUBMITTED TO THE ARCHITECT FOR REVIEW BY THE OR FLOOR. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE ADHESIVE ANCHORS IN CONCRETE SHALL BE HILTI HIT-500-SD AS STRUCTURAL ENGINEER PRIOR TO FABRICATION IN ACCORDANCE WITH THE FOOT. PROVIDE ADEQUATE SHORING AND/OR BRACING WHERE STRUCTURE HAS 8. THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BETWEEN SUPPORTS MANUFACTURED BY HILTI INC. (ICC ESR-2322) SPECIFICATIONS AND APPLICABLE CODES. THESE DRAWINGS SHALL BE NOT ATTAINED DESIGN STRENGTH. BY BUTTON PUNCHING. AVAILABLE ON THE JOB SITE PRIOR TO PLACING OF CONCRETE. 2 SPECIAL INSPECTION IN ACCORDANCE WITH TABLE 1705.3 OF THE IBC SHALL BE 14. THE CONTRACTOR SHALL VERIFY THE EXTENT AND LOCATIONS OF SITE UTILITIES PROVIDED FOR THE HIT-RE-500-SD ANCHOR INSTALLATIONS. 10. MILL TEST REPORTS FOR GRADE 60 BARS SHALL BE SUBMITTED PRIOR TO 9. PROVIDE FLASHING AND CLOSURE PLATES AT ENDS OF ALL UNITS, AROUND PRIOR TO EXCAVATION OR SHORING. SINCE THE SURVEY WAS BASED PLACEMENT OF CONCRETE. COLUMNS, AND AT ALL PERIMETER LOCATIONS REQUIRING CONCRETE. PRIMARILY ON PUBLIC RECORDS, THERE MAY BE DISCREPANCIES BETWEEN THE INSTALL ANCHORS PER THE INSTALLATION PROCEDURE DESCRIBED IN THE LOCATION INDICATED ON THE SITE SURVEY AND ACTUAL VERIFIED LOCATIONS. REPORT'S REFERENCED ABOVE WITH CARBIDE-TIPPED DRILL BITS CONFORMING 11. CONTINUOUS INSPECTION OF CONCRETE SHALL INCLUDE INSPECTION DURING 10. ALLOWABLE LOADS FOR DECK PER VERCO CATALOG (ICC ESR-1735P). IF THE ACTUAL FIELD VERIFIED LOCATION OF UTILITIES COULD RESULT IN A INSTALLATION OF REINFORCING STEEL. INSPECTION SHALL BE SCHEDULED CONFLICT WITH THE SHORING, THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY. SO THAT PLACEMENT OF REINFORCING STEEL, CONDUIT, SLEEVES, AND 4. USE NON-DESTRUCTIVE METHODS TO LOCATE EXISTING REINFORCING BARS. DO EMBEDDED ITEMS MAY BE CORRECTED PRIOR TO PLACEMENT OF OVERLYING 15. HEAVY EQUIPMENT, CRANES AND MATERIAL STOCKPILES SHALL NOT BE LOCATED NOT CUT EXISTING REINFORCING BARS WHICH MAY CONFLICT WITH ANCHOR GRIDS OR REINFORCING STEEL. ON OR ADJACENT TO SHORING UNLESS APPROVED BY THE STRUCTURAL ENGINEER. LOCATION UNLESS SPECIFICALLY APPROVED BY THE ENGINEER. 12. CONCRETE PROTECTION FOR REINFORCEMENT 16. CONTRACTOR SHALL COORDINATE SHORING WITH DRAWINGS OF RECORD TO INSURE 5. PROOF LOAD TESTS SHALL BE IN ACCORDANCE WITH MANUFACTURER AND CODE PROVISIONS FOR POCKETS, BLOCKOUTS, OFFSETS, STEPPED FOOTINGS AND ANY CAST-IN-PLACE CONCRETE (NON-PRESTRESSED). THE FOLLOWING OTHER ITEMS AFFECTED BY THE SHORING. MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT: 6. THESE TYPES OF ANCHORS ARE PERMITTED INDOORS WHERE AT LEAST 1 HOUR 17. AN UNDERGROUND SERVICE ALERT INQUIRY IDENTIFICATION NUMBER MUST BE FIRE RATING IS PROVIDED. MINIMUM COVER, IN. OBTAINED AT LEAST TWO WORKING DAYS BEFORE STARTING WORK WITH THIS PERMIT. TELEPHONE NO. 1-800-422-4133. FOR ANCHORS BY BRANDS OTHER THAN THAT LISTED ON THESE DRAWINGS, A. CONCRETE CAST AGAINST AND PERMANENTLY CONTRACTOR SHALL SUBMIT APPROVED TEST REPORTS FOR ENGINEER'S EXPOSED TO EARTH 18. STOCK PILING OR STORAGE OF MATERIAL ON OR NEAR SHORING BULKHEAD APPROVAL PRIOR TO INSTALLATION, FOR THE PARTICULAR LOCATION OF USE. IS NOT PERMITTED. B. CONCRETE EXPOSED TO EARTH OR WEATHER: 19. SEE ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS DRAINAGE AND WATERPROOFING. NO. 6 THROUGH NO. 11 BAR 1-1/2 NO 5 BAR, W31 OR D31 WIRE & SMALLER 20. DESIGN LOADS: C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS, WALLS, JOISTS: NO. 11 BAR & SMALLER BEAMS, COLUMNS: LIVE LOADS: 60 PSF + 25 PSF (PARTITION) EXISTING FLOOR PRIMARY REINFORCEMENT TIES, STIRRUPS, SPIRALS 21. WIND ANALYSIS PER SECTION 1609 OF THE 2012 IBC BASIC WIND SPEED (3 SECOND GUST) V_{ULT} = 110 MPH, EXPOSURE B, RISK CATEGORY \overline{II} 22. SEISMIC ANALYSIS PER SECTION 1613 OF THE 2012 IBC AND H-18-8 VA SEISMIC DESIGN REQUIREMENTS RISK CATEGORY II $S_s = 2.239, S_1 = 0.822$ $S_{DS} = 1.493 \quad S_{D1} = 0.822$ I_P = 1.0 SEISMIC DESIGN CATEGORY D NON-SPRINKLERED **CD2 SUBMISSION - ISSUED FOR CONSTRUCTION** Drawing Title Description Seals and Signatures ARCHITECT/ENGINEERS: CONSULTANTS: Office of 898CM2032 **GENERAL NOTES** VA Los Angeles National Cemetery, Construction Phase 1A and Facilities ARCHITECTURE 6080 CENTER DRIVE, SUITE 300 Los Angeles National Cemetery 950 South Sepulveda Boulevard Los Angeles, California 90049 Drawing Number LOS ANGELES, CA 90045 Approved: Project Director Management INTERIORS Tel 310-665-1536 **S2** Los Angeles, California 90071, USA Department of Veterans Affairs 06-10-2016 213-629-0100 F213-629-0070 VA FORM 08-6231















